R19 Web Date: 11/20/2012



35030 SE Douglas Street, Suite 210 Snoqualmie, WA 98065-9266 **206-296-6600** TTY Relay: 711

www.kingcounty.gov

For alternate formats, call 206-296-6600.

Road Standards Variance Request to the

County Road Engineer

Project Name:	Permitting File No.: VARR18-00099
Gunshy Manor	Toganitaring File From Vy (FCF) 000000
Project Address and Parcel Number:	Signature: Date: 1,7/10
20005 NE Union Hill Road, Redmond, WA 98053	Laina 0/ 6/11/19
Applicant/Design Engineer Name:	Engineering Firm Name: ESM Consulting Engineers, LLC
The Estate of Barbara J Nelson c/o William C. Nelson, Jr. Address:	Telephone:
16508 NE 79th St	253-838-6113
City, State, ZIP:	Permitting Engineer Initials:
Redmond, WA 98072	Route Application
☐ Check here if project engineering plans are approved an	d construction has begun.
your variance request. For a complete list of road variance Failure to provide all pertinent information may result in deleguest and applicable fee to the Department of Permitting Center at 35030 SE Douglas Street, Suite 210 in Snoqualr	and Environmental Review (Permitting) Permitting Service
REFER TO SECTION 1.08 OF THE KING C	COUNTY ROAD STANDARDS FOR VARIANCES
DESCRIPTION OF VARIANCE REQUEST: See A	ıttached
2) 2.06 3) 2.08 4) 2.10 5) 3.02	Rural Local Roadways; Private Roads; Cul-de-sacs, Islands and Hammerhead; Intersection and Low-Speed Curves; Concrete Sidewalks; School Access
JUSTIFICATION (see attachments, pages All	to):
AUTHORIZATION SIGNATURES: PERMITTING STAFF RECOMMENDATION	DEPARTMENT OF TRANSPORTATION AUTHORIZATION
	Approval Conditioned Approval Denied
	County Design Engineer: Date
Development Engineer/Designee: Date	County Road Engineer: Date
CONDITIONS OF APPROVAL:	1









June 20, 2019

Job No. 1359-001-007

Mr. Robert Eichelsdoerfer, PE. Senior Engineer King County Dept. Of Transportation 201 S Jackson St Seattle, WA 98104

And

Ms. Huey-Yi Sung, PE, Senior Engineer King County Dept. of Local Services. Permitting Division 35030 SE Douglas Street, Ste #210 Snoqualmie, WA 98065

RE:

Gunshy Manor Preliminary Plat - VARR18-0009

Revised Road Variance Requests to the 2016 King County Road Design and Construction Standards

Dear Robert and Huey:

On behalf of The Estate of Barbara J Nelson and the WCN GST Non-Exempt Marital Trust #2 (the "applicant") and pursuant to subsection D.5 of Section 1.13 (Variances) of the 2016 King County Road Design and Construction Standards (KCRDCS), ESM Consulting Engineers is submitting this revised letter and relevant exhibits to serve as the applicant's request for determinations and approvals of certain variances in relation to provisions of the KCRDCS for the proposed Gunshy Manor Preliminary Plat.

In early April 2019, the applicant and ESM Consulting Engineers attended a meeting with King County staff to discuss comments provided by the County on March 21, 2019. During this meeting, County staff provided support of the road variance application and expressed no concerns with the proposed variances. In addition, further correspondence with Robert, KCDOT Engineer, concluded agreement on the revised private-road boulevard near the entrance of the project site and within the critical area buffer, ESM also discussed with Huev, KCDOLS Engineer. the proposal to provide a sidewalk on one side of the boulevard section and the need for a crosswalk that meets sight distance for safety purposes.

This revised road variance request is provided to the County to clarify specific requests and to address additional road variances required, which are italicized in the next section of this letter. Please review the Gunshy Manor Preliminary Site Plan as a visual aid to represent these specific requests.

Road Variance Requests

The applicant requests the following determinations and variances in relation to one or more subsections of each of the following 2016 KCRDCS sections:

- (1) A determination by the Development Engineer for approval of a "curb" type roadway on a rural road within the boulevard section of the proposed Tract G, as supported in Section 2.01 Land Developments in Rural Areas and specified in subsection E of section 2.02 Rural Local Roadways;
- (2) Approval by the Development Engineer to allow a private rural subaccess roadway with thickened asphalt edge rather than with curb, pursuant to subsection A of Section 2.06 (Private Roads);
- (3) A determination by the County Road Engineer pursuant to subsection B of Section 2.06 (Private Roads) to allow 23 single-family residential lots to be served by the private road;
- (4) Approval by the County Road Engineer of a variance pursuant to subsection A4 of Section 2.08 (Cul-de-Sacs, Islands, and Hammerheads) for the length of the proposed ±2900-foot-long permanent cul-de-sac;
- (5) A determination by the County Road Engineer that the intersection spacing provisions of Section 2.10 Intersection and Low-Speed Curves are inapplicable under Section 2.06.G.8 regarding the proposed private road intersection with NE Union Hill Road;
- (6) Approval by the County Road Engineer to allow a concrete sidewalk on one side of the rural road within the boulevard section of the proposed Tract G, as specified in Section 3.02(1);
- (7) A consideration of following Figure 2-005, Extruded Curb Roadway, along a 60' long portion of NE Union Hill Road, as an alternative provision for safe School Access for students within the proposed development, as outlined in Section 3.09.

Background

The proposed Gunshy Manor preliminary plat is for 23 single-family lots arranged in a clustered layout to avoid impacts to the extensive critical areas of the site. The private road (Tract G) enters from NE Union Hill Road and extends into the project site, crossing Martin Creek and its buffer. The proposed cross-sections are provided to comply with KCC 21A.24.125, avoiding and minimizing impacts to critical areas, as well as to provide for adequate fire access as required in the 2016 KCRDCS.

This project proposes a boulevard section through the Martin Creek buffer that includes a gated entryway, two residential and fire apparatus paths for egress and ingress, center median, curb and gutter and a sidewalk on one side.



Beyond the boulevard section, the proposed Tract G contains a 48' rural subaccess private roadway with 28' of pavement to support two travel lanes and parking on one side, along with thickened edge and a 4' shoulder on both sides, with additional 6' of grade on both sides to meet the minimum standard for this type of road section. The entirety of Tract G extends approximately 1,500 feet to a midsection eyebrow cul-de-sac for large-vehicle turnaround relief and extends another approximately 1,400 feet to the end of the cul-de-sac where it terminates in a bulb.

1. "Curb" Type Roadway Request - 2.02(E) Rural Local Roadways

Subsection E of the 2016 KCRDCS Section 2.02 provides provision for the County Road Engineer or Development Engineer to approve a variance from a "shoulder" type roadway within Rural Local Roadways. In order to comply with KCC 21A.24.125 while also providing for the necessary features of a private and rural roadway, the boulevard section of Tract G proposes the use of a "curb" type roadway with a sidewalk on one side to minimize the footprint of the roadway within the Martin Creek crossing and buffer.

The use of the "curb" typing minimizes the permanent road improvements by minimum width of 3' along the length of the boulevard section, which totals over 1,300 SF of minimization while also providing a safe walking path for pedestrians. Section 2.01, Land Development in Rural Areas supports the use of "curb" typing within cluster subdivision such as this to support minimizing impacts to critical areas.

2. Rural Subaccess Private Roadway with Thickened Edge Variance - 2.06(A) Private Roads

Subsection A of Section 2.06 (Private Roads) expresses that the Development Engineer may allow a thickened edge asphalt with the provision that stormwater treatment will be adequate and safety uncompromised. The cross-section C-C proposed for the primary portion of Tract G includes 2' thickened edge on both sides of the road as opposed to curbing or immediate shoulder and ditch as depicted on Fig. 2-001 of King County Road Standard Figures.

Curbing is not within this section of the road as it is not consistent with the edges for a rural roadway. An immediate shoulder with a ditch is also not proposed because a storm drainage system, designed to meet the 2016 King County Stormwater Manual standards, is proposed through a series of piping within the roadway and detention facilities on the site to collect, treat and disperse stormwater.

Safety is not compromised with this design – pedestrian access is available along the 4' shoulders immediately adjacent to the thickened edge along the entire road, along with a crosswalk that connects to the sidewalk for the boulevard section. The width of the roadway as proposed, 28' of paving in total, provides room for 2 travel lanes and parking on the eastern side of the road.

Additionally, in view of the significant length of the proposed private road, having the road privately owned and maintained supports the intent of a private road because all the existing surrounding parcels already have means of existing access to a public road. The County will not be responsible for maintaining another public road through this proposal.



3. Determination to Allow 23 Single-Family Lots - 2.06(B) Private Roads

The proposal plans for the private road to serve all 23 of the proposed single-family lots within the clustered subdivision. KCRDCS Subsection B of Section 2.06 (Private Roads) allows up to 50 lots to be served by private roads when the entire length of a proposed private road system to the nearest public maintained road is considered, and when the County Road Engineer determines that the following criteria are met:

- 1. There is no opportunity for connecting to neighboring parcels or developments, or
- When there are physical barriers, zoning regulations, legal constraints or any other applicable restrictions that prohibits the connection to road stub-outs, easements, neighboring parcel(s), public roads, or rights of way.

In addition, there is no opportunity for connection of the subject development to existing roads on adjacent properties or developments because none of those properties have existing roads stubbed to the subject site. Further, steep slopes exist to the south and east, and wetlands and aquatic areas exist to the south and west. Also, the existing lots to the west of the north end of the proposed subdivisions site have been fully developed. In sum, these constraints provide no opportunity for connecting to neighboring parcels or developments.

4. Road Length Variance - 2.08(A4) Cul-De-Sacs, Islands and Hammerheads

Subsection A4 of KCRDCS Section 2.08 (Cul-De-Sacs, Islands, and Hammerheads) provides a general limit of 600 feet on a permanent cul-de-sac but provides for variance opportunity by the County Road Engineer. As proposed, the Gunshy Manor subdivision contains a tract for a private roadway (Tract G) with a cul-de-sac extending approximately 1,400 feet from centerline of the midsection eyebrow cul-de-sac to the center of the terminal bulb and roughly another 1,500 lineal feet for a total length of ±2,900 lineal feet.

However, because of (1) the site's elongated geometry from north to south and the barriers presented by the site's critical areas and (2) the lack of existing roads to the subject site from surrounding properties, there is no other reasonable layout alternative for a road to serve the proposed lots. In addition to the internal eyebrow cul-de-sac for emergency and service vehicle turnaround, individual driveways would allow turn-around opportunities along the length of the cul-de-sac as each proposed driveway will be longer than 30 feet long (while some over 100 feet long) and 16 feet in width to match KCRDCS standards.

Note that precedent exists for approval of a cul-de-sac in unincorporated King County longer than the length of the subject proposed cul-de-sac. For example, the "Ames Lake Hills" residential subdivision was approved by King County, recorded (King County Recording Number 9505151542), and developed with an overall cul-de-sac length of ±4,000 feet. (See the accompanying Ames Lake Hills cul-de-sac exhibit.)



5. <u>Determination That the Intersection Spacing Provisions Are Inapplicable to the Proposed Private Roadway - 2.10(B) Intersections and Low-Speed Curve</u>

The intersection spacing provisions of Section 2.10(B) are inapplicable to the subject proposed private road. On page number 6 of the accompanying "Gunshy Manor Site Access Analysis" memorandum prepared by Transpo Group, Transpo explains why, as can be seen in the following excerpt:

Intersection Spacing

The proposed access along NE Union Hill Road was analyzed relative to intersection spacing per *King County Road Design and Construction Standards – 2016.* Per King County Road Standards, intersection spacing is set for the highest classification of street involved, which for these intersections is NE Union Hill Road (a minor arterial), resulting in a recommended spacing of 500 feet between intersections. This is intended for public street connections with higher volumes. The proposed private access is located approximately 180 feet west of 199th Avenue NE. Both 199th Avenue NE and the proposed access are private roads intersecting NE Union Hill Road.

As both accesses are private roads, the minimum roadway spacing requirements noted in the standards are not applicable. The intersection spacing is a result of providing improved sight distance east of the proposed driveway along NE Union Hill Road. The private road of 199th Avenue NE serves 7 single-family homes and up to 25 homes via the proposed access. Low volumes are forecast at both of the proposed accesses with up to 21 trips during the weekday PM peak hour at the proposed access as shown above and approximately 7 trips during the weekday PM peak hour at the 199th Avenue NE access. The two private roads provide access to single family homes with low volumes of traffic and the 180 feet between the driveways will be sufficient.

As an additional justification for the requested determination, note that prior to the applicant's purchase of the parcel of land that has become the westernmost part of the site that abuts the south side of NE Union Hill Road, the planned NE Union Hill Road access point to the project site was substantially farther to the east, where there was less sight distance visibility available to and from the east around and behind the NE Union Hill Road curve. That said, the current proposal provides greater sight distance with the new access point location and minimal need for right-of-way clearing. (For additional information concerning sight distance, please refer to page numbers 5 and 6 of Transpo's November 29, 2018 Sight Access Analysis Memorandum).

6. Approval for Concrete Sidewalk on One Side of Rural Roadway - 3.02(1) Concrete Sidewalks

In addition to the aforementioned design alternatives and justifications proposed within Tract G, the proposed boulevard section contains a 5' wide sidewalk on one side. Section 3.02(1) calls for sidewalks on both sides when provided, unless otherwise allowed by the County Road Engineer.

In an effort to support safe pedestrian use, a crosswalk that meets sight-distance requirements is proposed at the southern end of the boulevard section for those walking along the eastern shoulder, as a walking path is not provided on the eastern side of the boulevard section. The provision for a sidewalk on one side of the boulevard section is to support the effort to minimize impacts to the Martin Creek crossing and buffer.



7. Consideration to use Extruded Curb as shown in Figure 2-005 - 3.09 School Access

Section 3.09 provides specifications for surfacing standards when school access is required as part of the development approval. Since a bus stop is already located within 50' of the proposed entryway for the project on NE Union Hill Road, school access improvements were not specifically required by the County or Lake Washington School District. However, in order to support new students that may reside in the proposed subdivision, this project proposes to provide a safe waiting area for students along Union Hill.

From the western edge of the proposed entryway improvements for the project site, an extruded curb will be installed at the edge of the existing pavement with a 5' wide crushed-surface walkway that extends 60' westward. At approximately 50' west of the entryway, an 8' wide crosswalk will be provided at the bus stop location. The applicant asks the County Road Engineer to consider and approve the use of design standards shown in Figure 2-005 along NE Union Hill for 60' in support of safe School Access. Please refer to the included Safe School Walkways Analysis exhibit to view correspondence with Lake Washington School District.

Summary

The requested determinations and the requested variance to the 2016 KCRDCS should be approved for the reasons set forth above and because they are in the public interest. The public interest will be served by issuance of the requested determinations and variance because (1) those decisions will allow the developable portion of the subject property to be developed for single-family residential lots off the proposed private road, (2) the design alternatives provide for safe vehicle and pedestrian accesses, and (3) impacts to existing critical areas will be reduced while allowing the project to achieve safety, functionality, fire protection, appearance and maintainability of the road network.

We look forward to working with you on this project. Please do not hesitate to contact me if you have any questions, comments or concerns.

Sincerely,

ESM CONSULTING ENGINEERS, LL.C.

LAURA BARTENHAGEN, P.E., LEED® AP

Principal

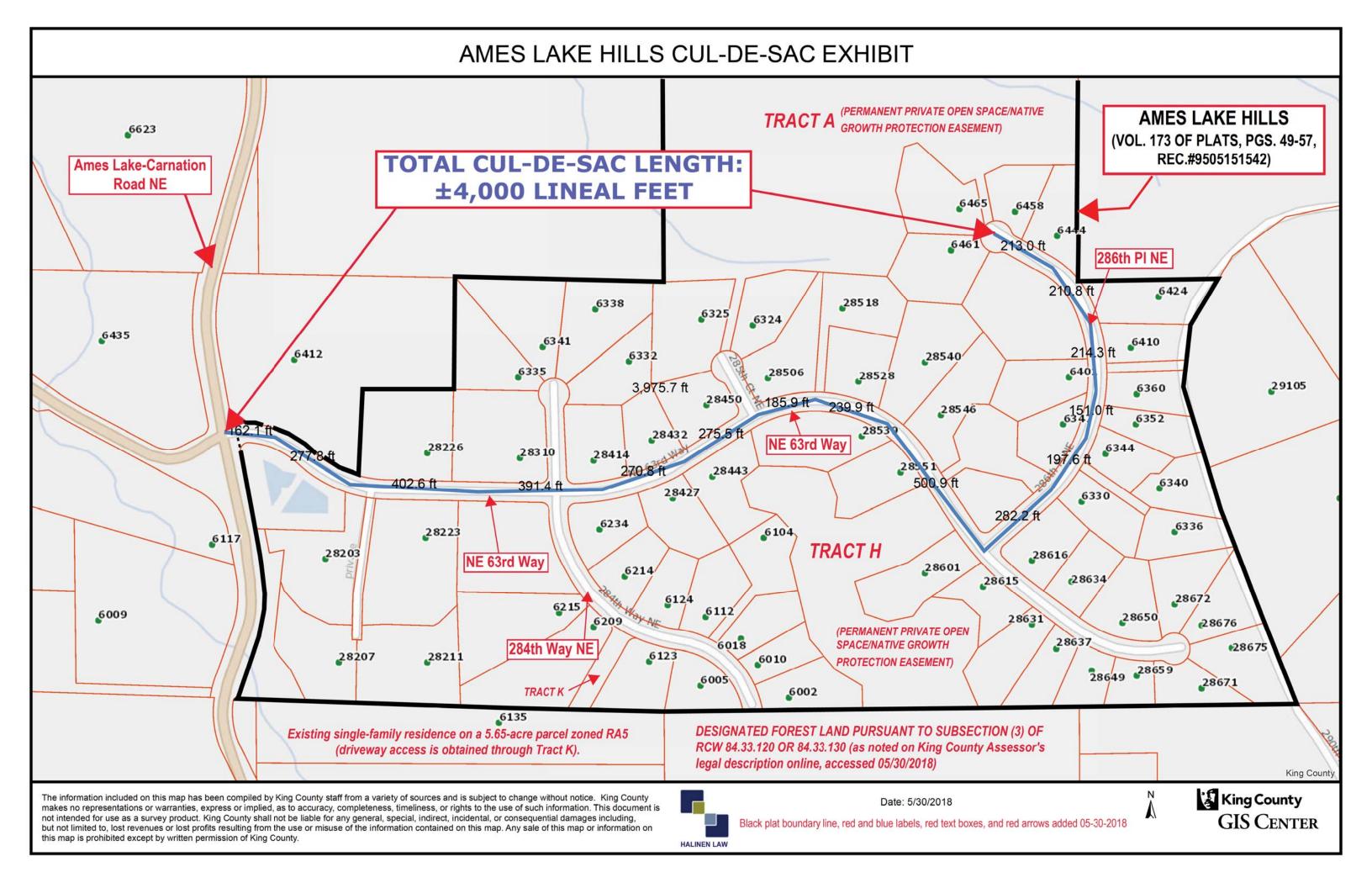
Enc: As Noted

cc: Buff Nelson, The Estate of Barbara J. Nelson

David Halinen, Halinen Law Offices

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MEMORANDUM

Date:	November 29, 2018	TG:	1.13203.04
To:	The Estate of Barbara Nelson C/O Buff Nelson		
From:	Dan McKinney, Jr. & Kassi Leingang, PE – Transpo Group		
cc:	Buff Nelson – Gunshy Manor		
Subject:	Gunshy Manor Site Access Analysis		

The following memorandum summarizes the operations of the site accesses to the proposed Gunshy Manor residential development and includes an overview of the project, project traffic volumes, and traffic operations.

Project Description

The project is located in unincorporated King County just east of the Redmond City limits and is bounded by NE Union Hill Road to the north (see Figure 1) and includes the development of up to 23 single family homes, with 3 existing homes in use being removed. Access to the 23 proposed homes is being provided via NE Union Hill Road approximately 1,000 feet east of the 196th Avenue NE/NE Union Hill Road roundabout in the northwest corner of the property. The driveway location was shifted to the west to enhance sight distances, which improves safety and reduces the amount of vegetation needing to be removed.



Figure 1 – Property Vicinity & Location

Traffic Volumes

The following section describes the existing and future without-project traffic volumes as well as estimates the traffic volumes associated with the proposed development.

Existing and Future Without-Project

A 7-day, 24-hour traffic volumes were collected along NE Union Hill Road in the vicinity of the primary proposed site access in order to capture the existing traffic volume profile in July/August 2018 (see Attachment A). The weekday mid-week (Tuesday-Thursday) average traffic volumes by time of day are shown in Figure 2 below.



Figure 2 - Existing NE Union Hill Road Traffic Volume Profile

As the figure shows, the weekday peak traffic volumes occur during the AM and PM peak periods at approximately 8:00 a.m. and 5:00 p.m. The weekday AM and PM peak hour traffic volumes at the site access intersections are discussed below.

Existing weekday AM and PM peak hour traffic counts were collected at the 196th Avenue NE/NE Union Hill Road intersection, northwest of the site in July 2018. Detailed existing traffic counts are provided in Attachment B. Future (2021) without-project traffic volumes were forecasted by applying an annual growth rate of 3 percent to existing traffic volumes.

Future With-Project

The traffic volumes associated with the proposed development were estimated based on trip rates in the Institute of Transportation Engineers (ITE) *Trip Generation Manual*, 10th Edition (2017). The development is estimated to generate approximately 189 net new vehicular weekday daily trips with 15 trips occurring during the weekday AM peak hour and 20 occurring during the weekday PM peak hour.



Table 1.	Estimated Weekday Vehicle	Trip Ge	neration						
				AM P	eak Hou	r Trips¹	PM P	eak Hou	r Trips
Single Fam	ily Detached Housing (LU #210)	Size	Daily Trips	In	Out	Total	In	Out	Total
Proposed		23 DU	227	4	13	17	14	9	23
Existing		3 DU	38	1	1	2	2	1	3
Net New		20 DU	189	3	12	15	12	8	20

Notes: DU= dwelling unit

These additional trips were distributed to and from the project site based on existing vehicle travel patterns and the proposed accesses. The trips were generally distributed with approximately 65 percent to/from the west, 20 percent to/from the east or southeast, and 15 percent to/from the north. The project trip distribution for vehicle trips is shown in Figure 3. The project trip assignment to each access is illustrated in Figure 3.

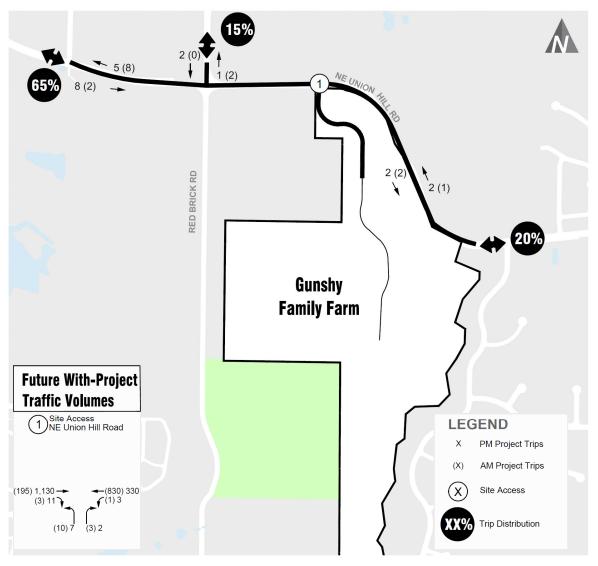


Figure 3 - Project Trip Distribution and Assignment and Future With-Project Traffic Volumes

^{1.} Trip generation based on ITE Trip Generation Manual (10th Edition, 2017).

The assigned project generated traffic was added to the future without-project weekday peak hour traffic volumes at the site access intersections. The resulting future (2021) with-project peak hour traffic volumes are shown in Figure 3.

Traffic Operations

The following section evaluates the operations of the site access intersections. The level of service of both accesses was evaluated. Additionally, the effective vehicle capacity was completed at the NE Union Hill Road site access.

Level of Service Analysis

The operational characteristics of an intersection are determined by calculating the intersection level of service (LOS). At unsignalized side-street, stop-controlled intersections, LOS is measured by the average delay on the worst-movement of the intersection. Traffic operations and average vehicle delay for an intersection can be described qualitatively with a range of levels of service (LOS A through LOS F), with LOS A indicating free-flowing traffic and LOS F indicating extreme congestion and long vehicle delays. Appendix B contains a detailed explanation of LOS criteria and definitions.

Weekday AM and PM peak hour traffic operations with the development of the project were evaluated at the site access intersections based on the procedures identified in the *Highway Capacity Manual* (2010) and were evaluated using *Synchro 9.1*. *Synchro 9.1* is a software program that uses *HCM* methodology to evaluate intersection LOS and average vehicle delays. Detailed LOS worksheets for both site access intersections are included in Attachment C. The weekday AM and PM peak hour operations at the site accesses are summarized in Table 1.

Table 2.	Future (2021) With-Proje	ect Weekday Al	M and PM	Peak Hour	Intersection	LOS Sun	nmary
		AI	M Peak Hou	ır	Р	M Peak Ho	ur
Intersection	l	LOS¹	Delay ²	WM ³	LOS	Delay	WM
1. Site Acces	ss/NE Union Hill Road	С	21	NB	D	33	NB

- 1. Level of Service (A F) as defined by the 2010 Highway Capacity Manual (HCM), Transportation Research Board.
- Average delay per vehicle in seconds.
- Worst movement reported for unsignalized two-way stop controlled intersections.

The primary site access along NE Union Hill Road is forecast to operate at LOS C and LOS D during the weekday AM and PM peak hours respectively with queues estimated to be up to 1 vehicle.

Effective Vehicle Capacity

In order to verify the LOS D or better operations determined in the LOS analysis above at the site access along NE Union Hill Road, the effective vehicle capacity was also evaluated. A combination of three factors go into calculating the effective vehicle capacity: the number of gaps¹ between vehicles, the headway² between vehicles as reflected by the length of time for the observed gaps, and observed courteous driver behavior. The consideration of these three factors is referred to as the effective vehicle capacity.

Data was collected for one week in July/August 2018 along NE Union Hill Road to determine the available vehicle capacity to accommodate vehicles exiting the project site (northbound right and northbound left movements from the site onto NE Union Hill Road). The amount of time required for a vehicle to turn out of the site driveway onto NE Union Hill Road was estimated based on the critical headway calculation per *Highway Capacity Manual* (HCM) 2010, Transportation Research

² Headway is defined as the time between two successive vehicles as they pass a point on the roadway measured from the same common feature of both vehicles (for example from the front bumper).



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¹ For this study, a gap is defined as the space between two vehicles.

Board.³ The time was estimated to be approximately 6 seconds for both the northbound left-turn and northbound right-turn movements.

The data was collected in various second intervals (e.g. 0 to 0.5 seconds, 0.5 to 1 second, 1 to 2 seconds, 2 to 4 seconds, 4 to 8 seconds, etc.). The number of gaps of 8 seconds or more were utilized for the analysis for both the northbound left-turn and northbound right-turn movements discussed below. The gaps and total time calculations are shown in Attachment D.⁴

- **Northbound Right-Turn**: The average minimum number of gaps during the AM peak period was 21 gaps during a 15-minute period. During the PM peak period (4 p.m. to 6 p.m.) the minimum number of gaps was 15 gaps during a 15-minute period.
- **Northbound Left-Turn**: The average minimum number of gaps during the AM peak period was 25 gaps during a 15-minute period. During the PM peak period (4 p.m. to 6 p.m.) the minimum number of gaps was 12 gaps during a 15-minute period.

As shown in Figure 3 above, the exiting volumes (i.e the northbound left and right-turn movements) are 13 and 9 vehicles during the weekday AM and PM peak hours. The gap analysis showed there are greater than 20 gaps every 15 minutes for both movements during the AM peak period and 12 or more gaps every 15 minutes for both movements during the PM peak period, both of which exceed the number of project trips in the respective time periods. Therefore, there is sufficient vehicle capacity along NE Union Hill Road to accommodate the anticipated peak hour vehicles exiting the site during the AM and PM peak hours.

Sight Distance Evaluation

Sight distance was evaluated at the proposed Gunshy Manor driveways consistent with King County sight distance standards⁵.

The 2012 Arterial Functional Class Unincorporated King County map classifies NE Union Hill Road as a minor arterial roadway. Per Section 2.02 of the *King County Road Design and Construction Standards (2016)*, the design speed of a minor arterial roadway is the posted speed plus 10 mph. In view of NE Union Hill Road's posted speed limit of 35 mph east of 196th Avenue NE, NE Union Hill Road's design speed is 45 mph.

Stopping sight distance is the distance needed for a vehicle to safely stop. Based on the design speed of 45 mph along NE Union Hill Road and the road's 9 percent downward grade, the required stopping sight distance for westbound traffic on NE Union Hill Road is 430 feet per *King County Road Design and Construction Standards (2016)*. For eastbound traffic, the required minimum stopping sight distance is 360 feet based on the design speed and the relatively flat roadway grade, which is less than 3 percent. The required stopping sight distance triangles east and west of the proposed driveway are shown in Attachments E and F, respectively. As shown in the attachments, the required stopping sight distance is met.

Entering sight distance is the distance needed for vehicles to not disrupt⁶ the traffic flow of the main roadway, rather than a distance for safety purposes (i.e. stopping sight distance). The recommended entering sight distance per *King County Road Design and Construction Standards* (2016), is 500 feet for both eastbound and westbound traffic based on a design speed of 45 mph. The recommended entering sight distance triangles east and west of the proposed driveway are shown in Attachments E and F, respectively.

⁶ Disrupt as defined per AASHTO means that "most major-road drivers should not need to reduce speed to less than 70 percent of their initial speed." (A Policy on Geometric Design of Highways and Streets, 6th Edition)



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³ HCM 2010 Section 19, equation 19-30.

⁴ The calculations were based on the mid-week data (Tuesday-Thursday) for the minimum gaps and max time of gaps that were too small for vehicles to exit the site onto NE Union Hill Road.

⁵ Per King County Road Design and Construction Standards – 2016.

The recommended entering sight distance west of the proposed driveway is met (see Attachment F). East of the driveway, the entering sight distance is currently obstructed by some vegetation and existing trees that are located within the right-of-way (see Attachment E). Attachment E highlights the location of trees near the curve. The developer will work with the County to remove necessary trees and clear underbrush or vegetation in the right-of-way to achieve adequate sight distance. Prior approval was granted by King County under GRDE16-0105, which allowed significantly more clearing than is now proposed.

Intersection Spacing

The proposed access along NE Union Hill Road was analyzed relative to intersection spacing per *King County Road Design and Construction Standards* – *2016*. Per King County Road Standards, intersection spacing is set for the highest classification of street involved, which for these intersections is NE Union Hill Road (a minor arterial), resulting in a recommended spacing of 500 feet between intersections. This is intended for public street connections with higher volumes. The proposed private access is located approximately 180 feet west of 199th Avenue NE. Both 199th Avenue NE and the proposed access are private roads intersecting NE Union Hill Road.

As both accesses are private roads, the minimum roadway spacing requirements noted in the standards are not applicable. The intersection spacing is a result of providing improved sight distance east of the proposed driveway along NE Union Hill Road. The private road of 199th Avenue NE serves 7 single family homes and up to 25 homes via the proposed access. Low volumes are forecast at both of the proposed accesses with up to 21 trips during the weekday PM peak hour at the proposed access as shown above and approximately 7 trips during the weekday PM peak hour at the 199th Avenue NE access. The two private roads provide access to single family homes with low volumes of traffic and the 180 feet between the driveways will be sufficient.

Summary

The Gunshy Manor residential development is located in unincorporated King County just east of the Redmond City limits and is bounded by NE Union Hill Road to the north and includes the development of up to 23 single family homes and removal of up to 3 existing single family homes. Access to the site is proposed along NE Union Hill Road. The development is estimated to generate approximately 189 net new vehicular weekday daily trips with 15 trips occurring during the weekday AM peak hour and 20 occurring during the weekday PM peak hour. The primary site access along NE Union Hill Road is forecast to operate at LOS C and LOS D during the weekday AM and PM peak hours respectively with queues estimated to be up to 1 vehicle. A gap analysis was performed and verified NE Union Hill Road has the capacity to accommodate the additional project trips.

⁹ The trip generation was estimated based on ITE's *Trip Generation Manual* (10th Edition, 2017) for Single Family Detached Housing (LU # 210), consistent with the proposed development.



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⁸ Per King County Road Design and Construction Standards – 2016: Section 2.06 G8– Private Roads "Not needed as public roads to meet the minimum road spacing requirements of these Standards".

Attachment A: 24-Hour Traffic Counts



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Date Range: 7/27/2018 - 8/2/2018

Date Range: 7/27/2018 - 8/2/2018

Site Code: 01

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687	399	138	797 797	375	6Z	10Z ZIÞ	948	130	208 9 77	778	69	424 474	719	29	02	001	20	601	591 291	34	424	940	1 /8	MA 00:5
986 684	189 887	138 138	926 692	9Z9 887	143	700,1	199 208	130	708 279	299 242	761	9 7 4	219 E77	128 128	747 286	181	901 27	324 324	163	121	720	229 629	141	MA 00:7 MA 00:8
022	999	214	733	019	223	477	Z99	202	208	689	213	902	967	211	420	270	120	242	334	117	417	203	211	MA 00:0
818	388	230	729	391	536	119	380	231	919	392	223	029	403	722	981	510	961	£78	735	916	989	388	248	MA 00:01
819	332	285	209	313	585	989	337	568	919	748	268	219	327	590	929	305	333	199	330	321	299	343	324	MA 00:11
029	314	326	089	326	324	699	315	344	029	301	698	989	279	998	989	364	321	<i>LL</i> 9	308	698	999	892	398	12:00 PM
689	275	392	919	892	348	999	282	585	769	274	598	229	792	322	619	997	323	969	568	362	702	762	907	N9 00∶1
702	253	677	717	243	<u></u>	689	724	432	002	192	6€₽	192	280	184	989	249	331	≯ 0∠	972	428	LZZ	598	209	Z:00 PM
226	567	629	1,028	323	907	∠ 96	162	929	756	282	929	922	285	769	524	223	301	743	262	8448	1,025	792	897	3:00 PM
1,263	697	7 66	1,262	292	۱'000	1,242	253	686	1,285	262	866	1,143	242	١06	7 49	226	348	249	172	175	1,163	289	478	4:00 PM
1,358	293	ا'90و	1,354	310	7†0°1	1,370	304	990'l	1,350	592	1,084	61E,1	724	390'l	919	961	321	282	529	323	901,1	526	820	6:00 PM
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1. Mid-week average includes data between Tuesday and Thursday.

%**†**†

6,851 5,443 12,294 4,745

%99

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%†9

%9†

4,109 8,854 3,972

Percent

MG 00:11

10:00 PM

M9 00:9

M9 00:8

M9 00:7

Total

Attachment B: Existing Peak Hour Traffic Counts

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Two-Hour Count Summaries 1,640 NE UNION HILL RD z» 932 = 700 = 0 196TH AVE NE **NE UNION HILL RD 196TH AVE NE** 0 = **218** PHF: 321 Peak Hour 2,287 0.94 785 18 26 196TH AVE NE **227** N 4 67 IE UNION HILL RD TOTAL WB SB $\frac{\mathsf{N}}{\mathsf{B}}$ ΕB 1,039 300 HV %: 2.8% 3.8% 1.0% 0.9% Count Period: Peak Hour: 0.72 0.97 0.98 뫆 0.79 0.94 Date: : Tue, Jul 31, 2018 : 4:00 PM to 6:00 PM : 4:00 PM to 5:00 PM 0

						count.	overall c	cles in	de bicv	ıt exclu	hicles bu	avv ve	lude he	nes inc	nlov v	summa	r count	Note: Two-hour count summary volumes include heavy vehicles but exclude bicycles in overall count
0	2,287	218	0	103	0	2	18	6	0	67	227	4	2	8	932	700	0	Peak Hour
0	4,471	414	0	239	0	4	40	8	0	132	434	6	3	12	1,879	1,300	0	Count Total
2,184	530	48	0	39	0	2	5	1	0	9	49	1	_	_	242	132	0	5:45 PM
2,209	526	55	0	40	0	0	4	_	0	14	45	0	0	_	214	152	0	5:30 PM
2,243	592	50	0	31	0	0	7	0	0	25	60	0	0	_	261	157	0	5:15 PM
2,217	536	43	0	26	0	0	6	0	0	17	53	_	0	_	230	159	0	5:00 PM
2,287	555	43	0	23	0	_	6	2	0	21	55	0	_	_	242	160	0	4:45 PM
0	560	52	0	20	0	0	Ν	_	0	20	54	_	0	_	258	151	0	4:30 PM
0	566	53	0	29	0	_	ω	2	0	14	55	2	0	Ŋ	228	174	0	4:15 PM
0	606	70	0	31	0	0	7	_	0	12	63	_	1	_	204	215	0	4:00 PM
		RT	Ħ	LT	UT	RT	Ŧ	ᄓ	UT	RT	TH	ᄓ	UT	RT	Ħ	LT	UT	
One Hour	Total		bound	Southbound			Northbound	North			Westbound	West			puno	Eastbound		Start
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Interval		Heavy	Heavy Vehicle Totals	Totals				Bicycles				Pedestrians (Cı		ossing Leg)	
Start	EB	WB	NB	SB	Total	EB	WB	NB	SB	Total	East	West	North	South	Total
4:00 PM	4	1	0	4	9	0	0	0	1	1	0	0	0	0	0
4:15 PM	5	_	_	_	∞	0	0	0	0	0	0	0	0	0	0
4:30 PM	_	_	0	2	4	_	0	0	0	1	0	0	0	0	0
4:45 PM	4	0	0	N	6	_	_	0	0	N	0	0	0	0	0
5:00 PM	З	0	0	σı	8	0	0	0	0	0	0	0	0	0	0
5:15 PM	2	0	0	0	N	0	0	0	0	0	0	0	0	0	0
5:30 PM	_	0	0	2	ω	2	0	0	0	N	0	0	0	0	0
5:45 PM	٦	0	0	4	5	0	0	0	0	0	0	0	0	0	0
Count Total	21	3	1	20	45	4	1	0	1	6	0	0	0	0	0
Peak Hour	14	3	_	9	27	2	1	0	1	4	0	0	0	0	0

www.idaxdata.com

Two-Hour Count Summaries 1,593 259 NE UNION HILL RD z» 142 = 108 = N 196TH AVE NE **NE UNION HILL RD 196TH AVE NE** 0 = 901 PHF: Peak Hour 1,993 0.85 209 20 36 196TH AVE NE **680** 0 <u>∞</u> IE UNION HILL RD TOTAL WB SB $\frac{\mathsf{N}}{\mathsf{B}}$ ΕB **1**79√ 762 HV %: 2.5% 0.0% 0.7% 8.9% Count Period: Peak Hour: **PHF** 0.71 0.64 0.80 0.94 0.85 Date: Tue, Jul 31, 2018 7:00 AM to 9:00 AM 8:00 AM to 9:00 AM 0 100

	NE NE	NE UNION HILL RD	HILL	RD	NE	OINU	NE UNION HILL RD	ď	1	96TH /	196TH AVE NE		1	196TH AVE NE	VE NE]
Start		Eastbound	ound			West	Westbound			Northbound	ound			Southbound	ound		Total	One Hour
	UT	디	Ŧ	RT	UT	ᄓ	로	RT	П	ᄓ	코	RT	UT	ᄓ	Ŧ	RT		9
7:00 AM	0	14	26	0	0	0	130	14	0	0	4	0	0	6	0	138	332	0
7:15 AM	0	31	31	_	0	0	138	12	0	0	0	0	0	Ŋ	0	176	394	0
7:30 AM	_	22	27	0	0	_	148	17	0	_	2	0	0	7	0	199	425	0
7:45 AM	0	21	37	_	0	_	172	17	0	_	ω	0	0	6	ω	198	460	1,611
8:00 AM	0	21	35	0	0	_	141	14	0	_	2	0	0	Ŋ	_	202	423	1,702
8:15 AM	0	22	24	_	0	0	140	14	0	ω	ω	2	0	7	2	227	445	1,753
8:30 AM	0	22	37	6	0	0	189	25	0	ω	51	ω	0	15	0	233	538	1,866
8:45 AM	2	43	46	0	0	0	210	28	0	3	10	1	0	4	1	239	587	1,993
Count Total	3	196	263	9	0	ω	1,268	141	0	12	29	6	0	55	7	1,612	3,604	0
Peak Hour	2	108	142	7	0	1	680	81	0	10	20	6	0	31	4	901	1,993	0
Note: Two-hour count summary volumes include heavy vehicles but exclude bicycles in overall count.	r count	summa	ry volur	nes inci	ude he	avy veł	nicles bu	t exclud	де Бісус	des in c	verall c	ount.						

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	Note: I wo-hour count summary
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Interval		Heavy	Heavy Vehicle Totals	Totals				Bicycles				Pedestrians (Cı		ossing Leg)	
Start	EB	WB	NB	SB	Total	EB	WB	NB	SB	Total	East	West	North	South	Total
7:00 AM	2	1	0	ω	6	0	0	0	0	0	0	2	0	0	2
7:15 AM	10	_	0	4	15	0	2	0	0	2	0	_	0	0	1
7:30 AM	5	2	_	_	9	0	2	0	0	2	0	_	0	0	1
7:45 AM	5	4	0	_	10	0	_	0	0	1	0	_	0	0	1
8:00 AM	4	_	0	7	12	0	0	0	0	0	0	0	0	0	0
8:15 AM	2	ω	0	0	51	0	0	0	_	1	0	0	0	0	0
8:30 AM	4	0	0	7	11	0	0	0	0	0	0	0	0	0	0
8:45 AM	13	1	0	9	23	1	0	0	0	1	0	0	0	0	0
Count Total	45	13	1	32	91	1	5	0	1	7	0	5	0	0	5
Peak Hour	23	5	0	23	51	_	0	0	1	2	0	0	0	0	0

Attachment C: LOS Worksheets

Intersection						
Int Delay, s/veh	0.3					
			VA/DI	MOT	ND	NDD
Movement	EBT	EBR	WBL	WBT	NBL	NBR
Lane Configurations	₽			-4	W	
Traffic Vol, veh/h	195	3	1	830	10	3
Future Vol, veh/h	195	3	1	830	10	3
Conflicting Peds, #/hr	0	0	0	0	0	0
	Free	Free	Free	Free	Stop	Stop
RT Channelized	-	None	-	None	-	None
Storage Length	-	-	-	-	0	-
Veh in Median Storage,	# 0	-	-	0	0	-
Grade, %	0	-	-	0	0	-
Peak Hour Factor	85	85	85	85	85	85
Heavy Vehicles, %	9	9	1	1	0	0
Mvmt Flow	229	4	1	976	12	4
		•	•	0.0		•
	ajor1		Major2		Minor1	
Conflicting Flow All	0	0	233	0	1210	231
Stage 1	-	-	-	-	231	-
Stage 2	-	-	-	-	979	-
Critical Hdwy	-	-	4.11	-	6.4	6.2
Critical Hdwy Stg 1	_	-	-	-	5.4	-
Critical Hdwy Stg 2	_	-	_	-	5.4	_
Follow-up Hdwy	_	_	2.209	_	3.5	3.3
Pot Cap-1 Maneuver	-	-	1340	_	204	813
Stage 1	_	_	-	_	812	-
Stage 2	_	_	_	_	367	_
Platoon blocked, %	_			_	001	
Mov Cap-1 Maneuver	_	_	1340	-	204	813
Mov Cap-1 Maneuver	-	-	1340	<u>-</u>	204	013
		-	-			
Stage 1	-	-	-	-	812	-
Stage 2	-	-	-	-	366	-
Approach	EB		WB		NB	
HCM Control Delay, s	0		0		20.5	
HCM LOS			- 0		20.0 C	
TIOWI LOO					J	
Minor Lane/Major Mvmt	١	NBLn1	EBT	EBR	WBL	WBT
Capacity (veh/h)		247	-	-	1340	-
HCM Lane V/C Ratio		0.062	-		0.001	-
HCM Control Delay (s)		20.5	-	-	7.7	0
HCM Lane LOS		С	-	-	Α	A
HCM 95th %tile Q(veh)		0.2	_	_	0	-
/ (1011)		J.L			J	

Transpo Group Synchro 9 Report

New New
Movement
Lane Configurations
Lane Configurations
Traffic Vol, veh/h 1130 11 3 330 7 2 Future Vol, veh/h 1130 11 3 330 7 2 Conflicting Peds, #/hr 0 0 0 0 0 0 Sign Control Free Free Free Free Free Free Stop Stop RT Channelized - None - - 0 0 - -
Future Vol, veh/h
Conflicting Peds, #/hr O O O O O O O Sign Control Free Free Free Free Free Free Stop Stop RT Channelized - None - None - None O O O O O O O O O
Sign Control Free Free Free Free Free Stop Stop RT Channelized None
RT Channelized - None - None - None Storage Length 0 0 0 - - Veh in Median Storage, # 0 0 0 0 - - 0 0 0 - - Grade, % 0 0 0 0 - - 0 0 0 - - Peak Hour Factor 94 94 94 94 94 94 94 94 94 94 94 94 Heavy Vehicles, % 1 1 1 1 1 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0
Storage Length
Veh in Median Storage, # 0 - - 0 0 - Grade, % 0 - - 0 0 - Peak Hour Factor 94 94 94 94 94 94 Heavy Vehicles, % 1 1 1 1 0 0 Mwrit Flow 1202 12 3 351 7 2 Major/Minor Major1 Major2 Minor1 Minor1 C 0 0 1214 0 1565 1208
Grade, % 0 - - 0 0 - Peak Hour Factor 94
Peak Hour Factor 94
Major/Minor
Momental Major/Minor Major Major Major Major Minor Minor Major Minor
Major/Minor Major1 Major2 Minor1 Conflicting Flow All 0 0 1214 0 1565 1208 Stage 1 - - - 1208 - Stage 2 - - - 1208 - Critical Hdwy - - 4.11 - 6.4 6.2 Critical Hdwy Stg 1 - - - 5.4 - Critical Hdwy Stg 2 - - - 5.4 - Follow-up Hdwy - 2.209 - 3.5 3.3 Pot Cap-1 Maneuver - 578 - 124 225 Stage 1 - - - 286 - Stage 2 - - - 713 - Mov Cap-1 Maneuver - - 578 - 123 225 Mov Cap-2 Maneuver - - - - 286 - Stage 2 <td< td=""></td<>
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Platoon blocked, % - - - Mov Cap-1 Maneuver - - 578 - 123 225 Mov Cap-2 Maneuver - - - - 123 - Stage 1 - - - - 286 - Stage 2 - - - - 709 - Approach EB WB NB NB HCM Control Delay, s 0 0.1 33.2 - HCM LOS D D D Minor Lane/Major Mvmt NBLn1 EBT EBR WBL WBT Capacity (veh/h) 137 - - 578 - HCM Lane V/C Ratio 0.07 - 0.006 -
Mov Cap-1 Maneuver - - 578 - 123 225 Mov Cap-2 Maneuver - - - - 123 - Stage 1 - - - - 286 - Stage 2 - - - - 709 - Approach EB WB NB NB HCM Control Delay, s 0 0.1 33.2 - HCM LOS D D - - 5 - Minor Lane/Major Mvmt NBLn1 EBT EBR WBL WBT Capacity (veh/h) 137 - - 578 - HCM Lane V/C Ratio 0.07 - - 0.006 -
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Mov Cap-2 Maneuver - - - 123 - Stage 1 - - - - 286 - Stage 2 - - - - 709 - Approach EB WB NB HCM Control Delay, s 0 0.1 33.2 HCM LOS D Minor Lane/Major Mvmt NBLn1 EBR WBL WBT Capacity (veh/h) 137 - - 578 - HCM Lane V/C Ratio 0.07 - - 0.006 -
Stage 1 - - - 286 - Stage 2 - - - 709 - Approach EB WB NB HCM Control Delay, s 0 0.1 33.2 HCM LOS D Minor Lane/Major Mvmt NBLn1 EBT EBR WBL WBT Capacity (veh/h) 137 - - 578 - HCM Lane V/C Ratio 0.07 - 0.006 -
Stage 2 - - - - 709 - Approach EB WB NB HCM Control Delay, s 0 0.1 33.2 HCM LOS D Minor Lane/Major Mvmt NBLn1 EBT EBR WBL WBT Capacity (veh/h) 137 - - 578 - HCM Lane V/C Ratio 0.07 - - 0.006 -
Approach EB WB NB HCM Control Delay, s 0 0.1 33.2 HCM LOS D Minor Lane/Major Mvmt NBLn1 EBT EBR WBL WBT Capacity (veh/h) 137 - - 578 - HCM Lane V/C Ratio 0.07 - - 0.006 -
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Minor Lane/Major Mvmt NBLn1 EBT EBR WBL WBT Capacity (veh/h) 137 - - 578 - HCM Lane V/C Ratio 0.07 - - 0.006 -
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HCM Lane V/C Ratio 0.07 0.006 -
HCM Control Delay (s) 33.2 11.3 0
HCM Lane LOS D B A
HCM 95th %tile Q(veh) 0.2 0 -

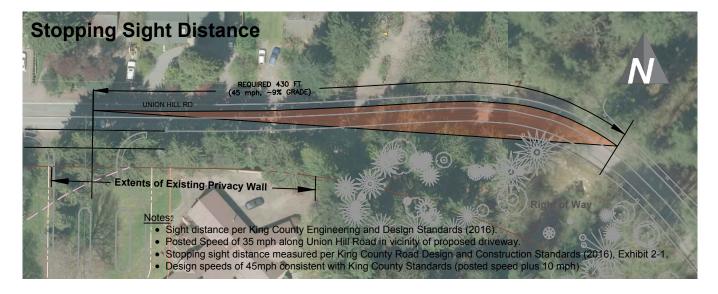
Attachment D: Gap Analysis

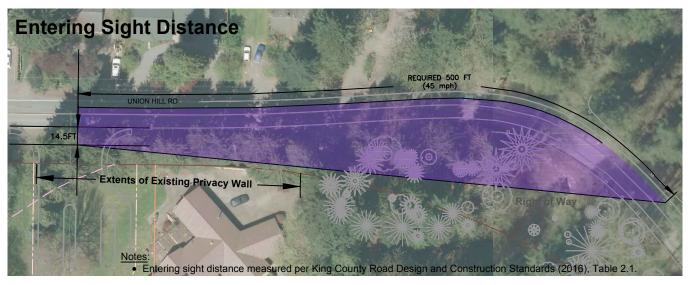
NORTHBOUND LEFT - uses the combined gap data as it crosses both the eastbound and westbound approaches 8 seconds or more

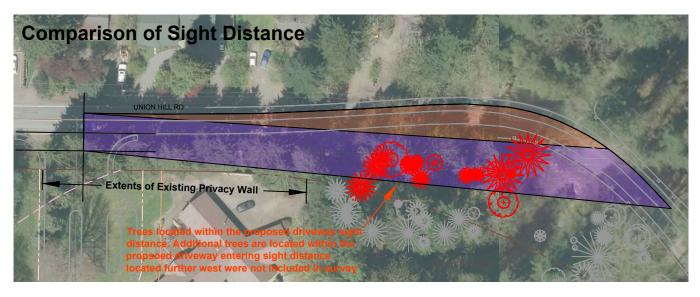
	Tuesday, July 31, 2018	Wednesday, August 1, 2018	Thursday, August 2, 2018	Minimum	
0700	31	31	30	30	
0715	31	30	29	29	
0730	26	30	33	26	
0745	28	36	31	28	
0800	29	28	29	28	
0815	29	24	25	24	
0830	18	21	18	18	
0845	21	24	18	18	
Minimum	18	21	18	25 Avera	rage
1600	18	24	16	16	
1615	21	17	16	16	
1630	9	10	20	9	
1645	18	18	14	14	
1700	7	10	4	4	
1715	16	13	13	13	
1730	13	9	14	9	
1745	11	13	16	11	
Minimum	7	9	4	12 Avera	rage

NORTHBOUND RIGHT - uses the eastbound gap data as it enters only the eastbound approach 8 seconds or more

Time	Tuesday, July 31, 2018	Wednesday, August 1, 2018	Thursday, August 2, 2018	Minimum
0700	22	14	20	14
0715	26	23	16	16
0730	22	27	22	22
0745	19	22	24	19
0800	20	27	24	20
0815	26	24	26	24
0830	23	30	23	23
0845	26	27	27	26
Minimum	19	14	16	21 Ave
1600	22	25	21	21
1615	26	22	16	16
1630	14	14	24	14
1645	21	22	19	19
1700	15	18	7	7
1715	20	16	21	16
1730	12	11	21	11
1745	13	18	19	13
Minimum	12	11	7	15 Ave



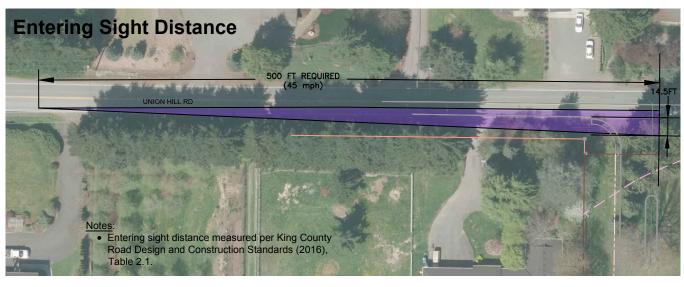




Sight Distance East of Proposed NE Union Hill Rd Driveway ATTACHMENT





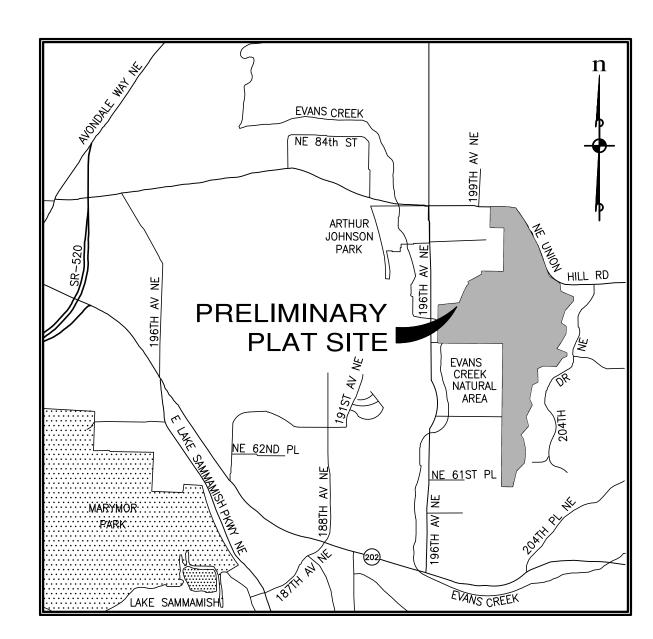


PRELIMINARY PLAT PLANS

for

GUNSHY MANOR

prepared for



Vicinity Map

SITE DATA

20005 NE UNION HILL RD REDMOND, WA 98053

PARCEL NUMBERS: 082506-9067, 9013, 9102, 9103, 9104, 9105

SITE AREA GROSS: 5,081,132 S.F. = 116.65 AC.

(GROSS MINUS AND BUFFERS):

1,122,804 S.F. = 25.78 AC.

COMP. PLAN DESIGNATION:

ZONING:

RURAL

EXISTING USE: FARM WITH SINGLE-FAMILY HOMES AND OUTBUILDINGS

RA-5, RA-5-P

23-LOT SINGLE-FAMILY RESIDENTIAL CLUSTER SUBDIVISION

DRAINAGE BASIN: EVANS CREEK

RA- 5 DEVELOPMENT STANDARDS (KCC 21A.12.030)

BASE DENSITY: 0.2 DU/ACRE

 $0.2 \times 116.65 \text{ AC.} = 23 \text{ LOTS}$ MAX. NO. OF LOTS:

MINIMUM LOT AREA: 0.75 AC. PROPOSED WITH CLUSTERING (KCC 21A.14.040.B.)

MINIMUM LOT WIDTH:

BUILDING SETBACKS:

10' INTERIOR: 20' GARAGE

BASE BUILDING HEIGHT: 40' MAX. BUILDING HEIGHT: 75' MAX. LOT IMPERVIOUS COVERAGE PERCENTAGE: 20%

UTILITY PROVIDERS

SEWAGE DISPOSAL: ON-SITE SEWAGE SYSTEMS WATER: UNION HILL WATER ASSOCIATION POWER: PUGET SOUND ENERGY GAS: PUGET SOUND ENERGY AND INTERNET: FRONTIER COMMUNICATIONS SCHOOL: LAKE WASHINGTON SCHOOL DISTRICT #414

FIRE: KING COUNTY FIRE DISTRICT #34

(425) 881-7831

PLANNER/ENGINEER/SURVEYOR

ESM CONSULTING ENGINEERS, L.L.C. 33400 8TH AVE S, STE 205 FEDERAL WAY, WA 98003 (253) 838-6113 CONTACT: ERIC LaBRIE, A.I.C.P. LAURA BARTENHAGEN, PE ZACK LENNON, PLS

WILLIAM C. NELSON, JR.

GEOTECH ENGINEER

ASSOCIATED EARTH SCIENCES, INC. 911 5TH AVE, STE 100 KIRKLAND, WA 98033 (253) 827-7701 CONTACT: MATT MILLER, PE

WETLAND/WILDLIFE BIOLOGIST

TALASAEA CONSULTANTS 15020 BEAR CREEK RD NE WOODINVILLE, WA 98077 (425) 861-7550 CONTACT: BILL SHIELS JENNIFER MARRIOTT, PWS

TRAFFIC ENGINEER

TRANSPO GROUP 12131 113TH AVE NE, STE 203 KIRKLAND, WA 98034 (425) 821-3665 CONTACT: DAN McKINNEY, JR. KASSI LEINGANG, PE

CULTURAL RESOURCES

ENVIRONMENTAL SCIENCE ASSOCIATES 5309 SHILSHOLE AVE NW, STE 200 SEATTLE, WA 98107 (206) 789-9658 CONTACT: MICHAEL MUSCARI, PWS

WASTEWATER ENGINEER

ADC WASTEWATER ENGINEERING 729 COURT C, TACOMA, WA 98402 (253) 203-1200 CONTACT: ROCKY ANDERSON

prepared by



33400 8th Ave S, Suite 205 FEDERAL WAY, WASHINGTON 98003 Phone: (253) 838-6113



ESM JOB NO. 1359-001-007 SHEET 1 OF

SHEET NO.	DRAWING NO.	DESCRIPTION
1	PP-01	COVER SHEET
2	PP-02	EXISTING CONDITIONS
3	PP-03	PRELIMINARY SITE PLAN
4	PP-04	PRELIMINARY SITE PLAN
5	PP-05	PRELIMINARY GRADING PLAN
6	PP-06	PRELIMINARY UTILITY PLAN
7	PP-07	PRELIMINARY ROADWAY PROFILE

LEGAL DESCRIPTION OF ALL CURRENT PARCELS **ENCOMPASSING THE PRELIMINARY PLAT SITE**

PARCEL 1

LOT 1 OF KING COUNTY BOUNDARY LINE ADJUSTMENT NO. BLAD 13-0001, AS RECORDED UNDER RECORDING NO. 20130610900001, RECORDS OF KING COUNTY AUDITOR;

PARCEL 2:

LOT 2 OF KING COUNTY BOUNDARY LINE ADJUSTMENT NO. BLAD 13-0002, AS RECORDED UNDER RECORDING NO. 20130610900002, RECORDS OF KING COUNTY AUDITOR;

PARCEL 3:

UNDER RECORDING NO. 20190508900002, RECORDS OF KING COUNTY AUDITOR; PARCEL 4:

LOT B OF KING COUNTY BOUNDARY LINE ADJUSTMENT NO. BLAD 18-0056, AS RECORDED

LOT 4 OF KING COUNTY BOUNDARY LINE ADJUSTMENT NO. BLAD 13-003, AS RECORDED UNDER RECORDING NO. 20130610900003, RECORDS OF KING COUNTY AUDITOR;

PARCEL 5:

LOT 5 OF KING COUNTY BOUNDARY LINE ADJUSTMENT NO. BLAD 13-0003, AS RECORDED UNDER RECORDING NO. 20130610900003, RECORDS OF KING COUNTY AUDITOR;

PARCEL 6:

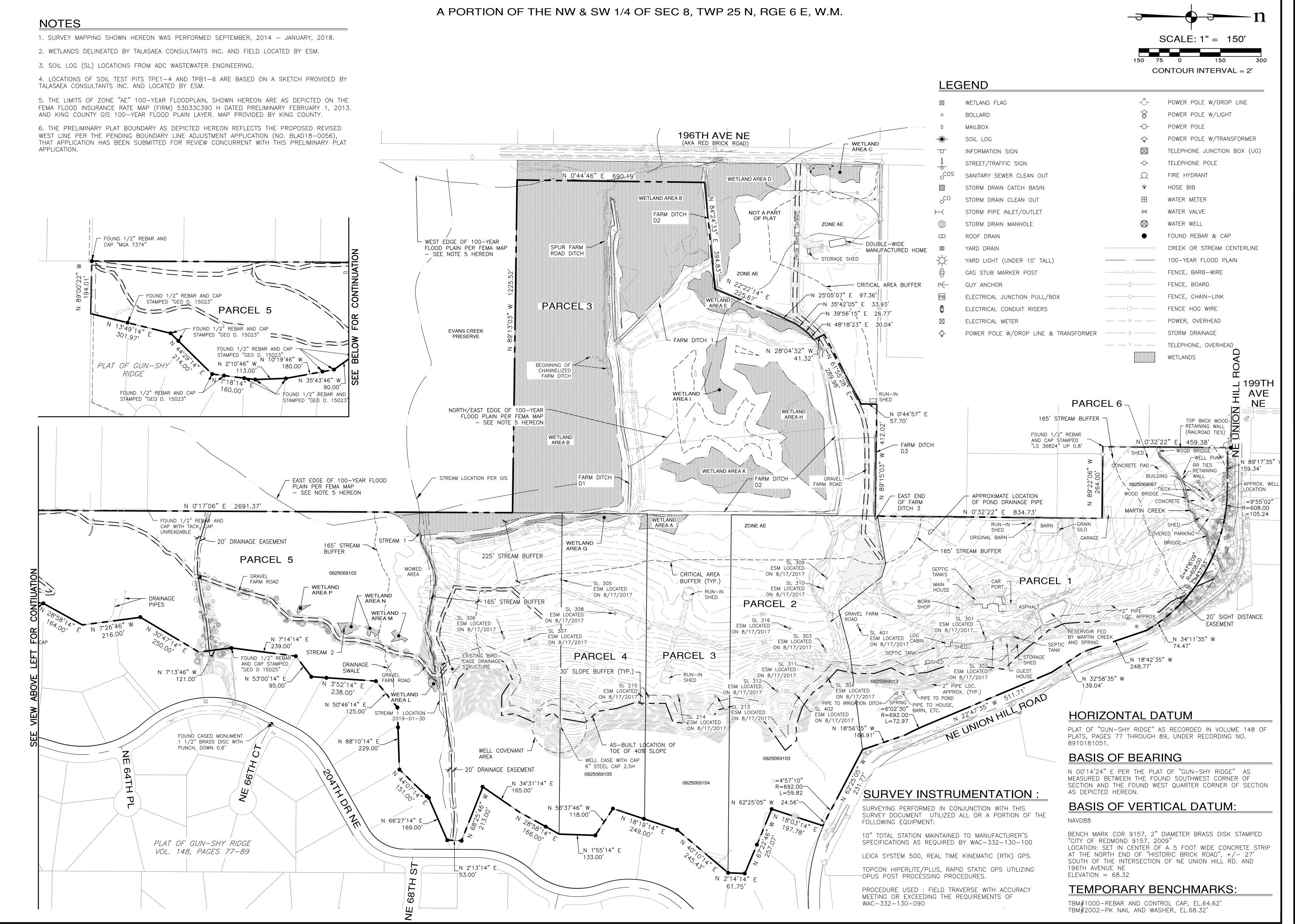
THE EAST 264 FEET OF THE EAST HALF OF THE FOLLOWING DESCRIBED PROPERTY: THE NORTH HALF OF THE NORTH HALF OF THE NORTHWEST QUARTER OF THE NORTHWEST QUARTER OF SECTION 8, TOWNSHIP 25 NORTH, RANGE 6 EAST, W.M., IN KING COUNTY, WASHINGTON;

EXCEPT THE WEST 30 FEET THEREOF CONVEYED TO THE KING COUNTY FOR ROAD BY DEED RECORDED UNDER RECORDING NO. 713244;

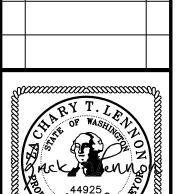
ALSO EXCEPT THAT PORTION LYING WITHIN NORTHEAST 80TH STREET; AND

ALSO EXCEPT THAT PORTION CONVEYED TO KING COUNTY BY DEED RECORDED UNDER RECORDING NO. 9411181045.

TOGETHER WITH THE EASTERLY 264 FEET OF THE NORTH HALF OF THE SOUTH HALF OF THE NORTH HALF OF THE NORTHWEST QUARTER OF THE NORTHWEST QUARTER OF SECTION 8, TOWNSHIP 25 NORTH, RANGE 6 EAST, W.M., IN KING COUNTY, WASHINGTON. SITUATE IN THE COUNTY OF KING, STATE OF WASHINGTON.



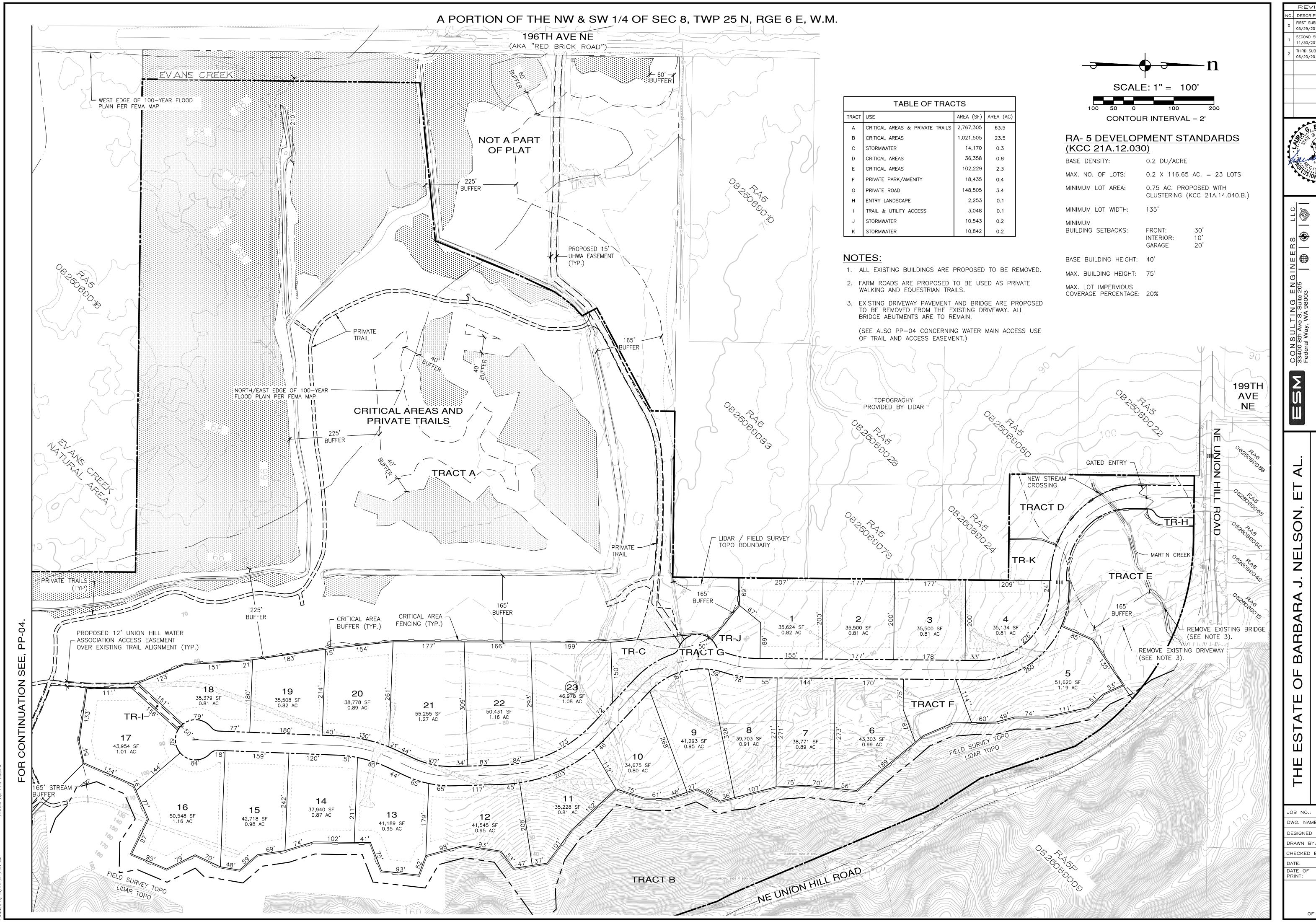
DESCRIPTION/DATE FIRST SUBMITTAL SECOND SUBMITTAL 11/30/2018 THIRD SUBMITTAL 06/20/2019



JOB NO.: 1359-001-00 DWG. NAME: DESIGNED BY: DRAWN BY: 06/20/2019

CHECKED BY: DATE OF

OF SHEETS

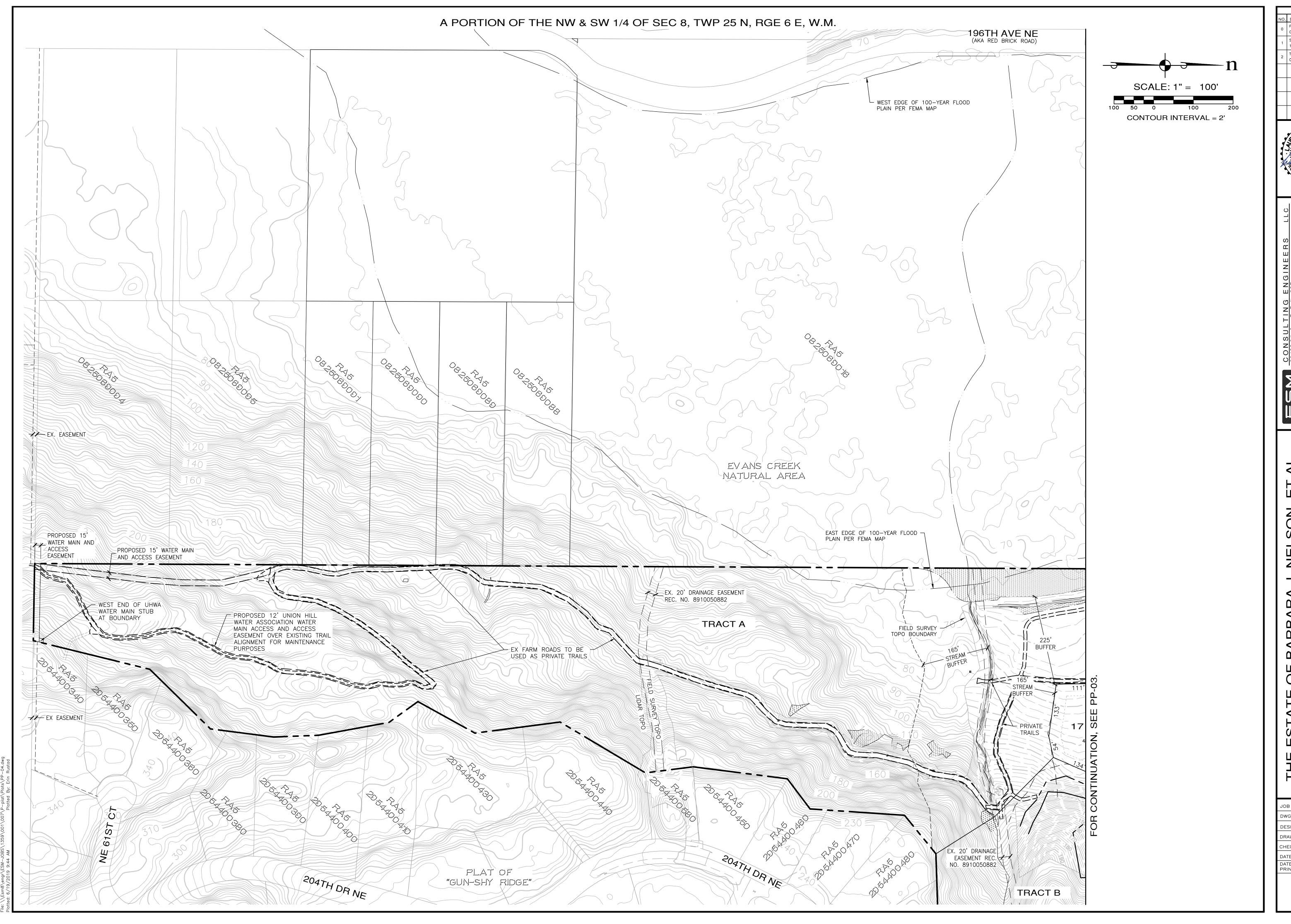


FIRST SUBMITTAL 05/29/2018 SECOND SUBMITTAL 11/30/2018 THIRD SUBMITTAL 06/20/2019

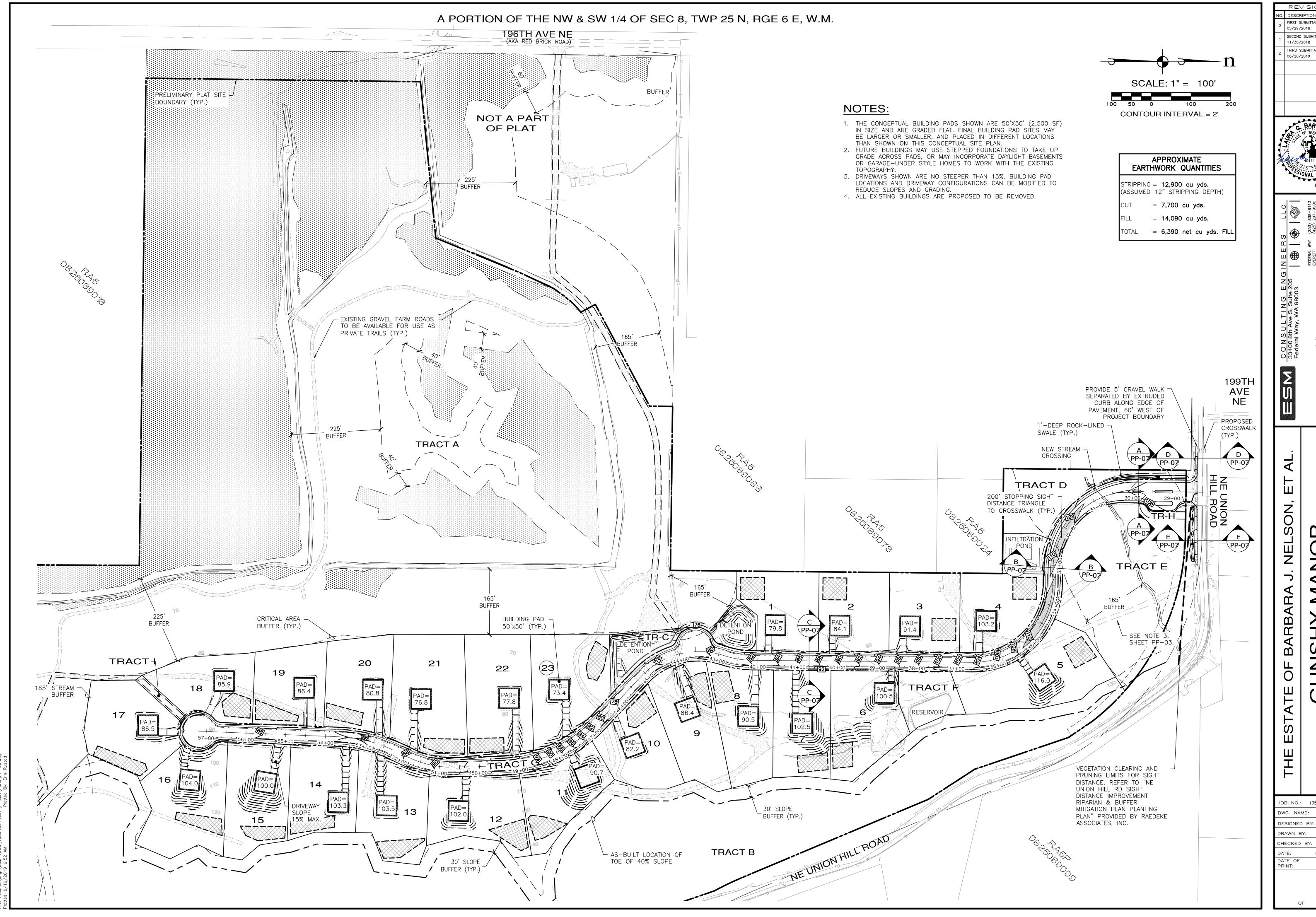


DWG. NAME: DESIGNED BY:

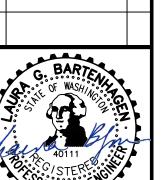
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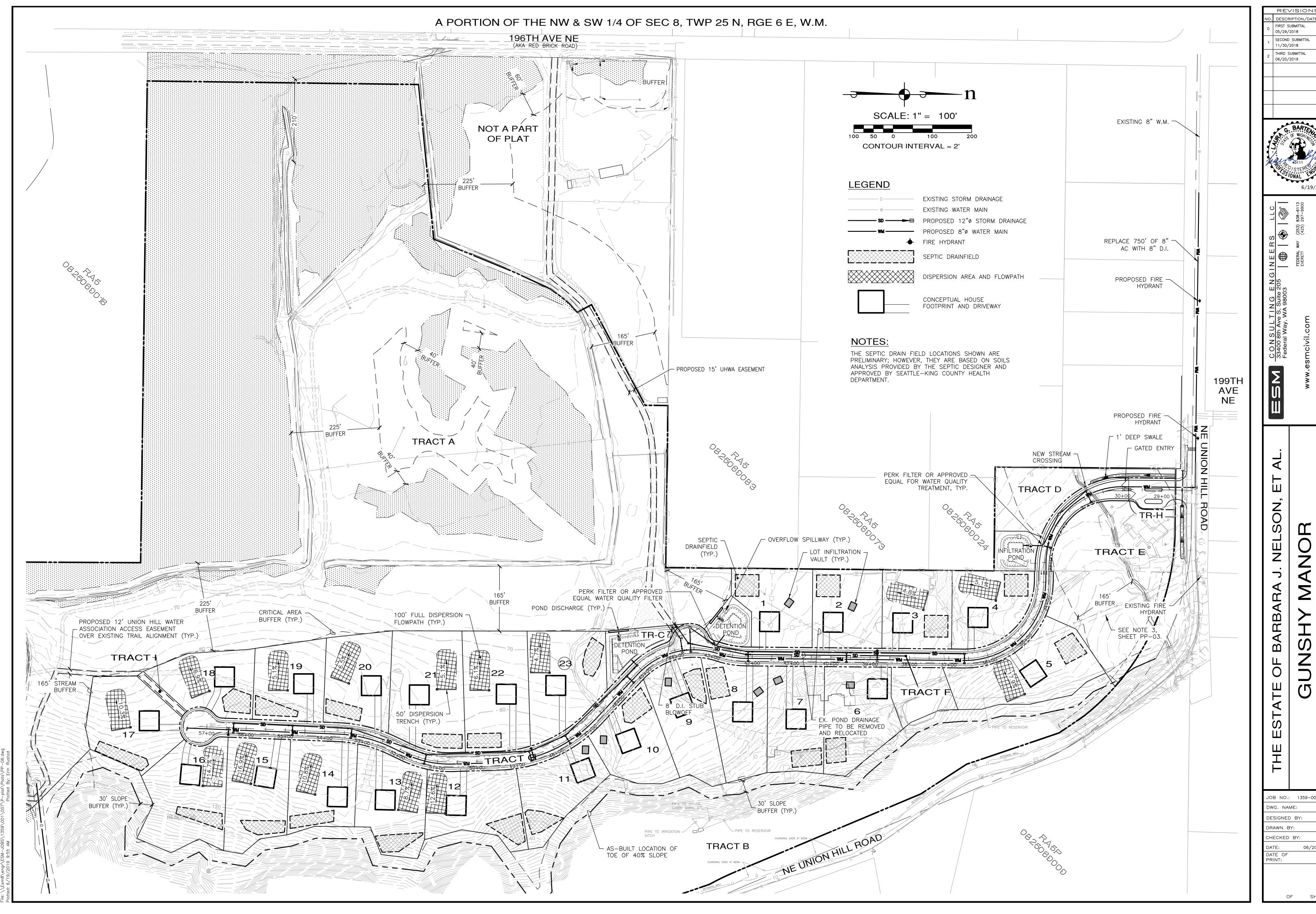


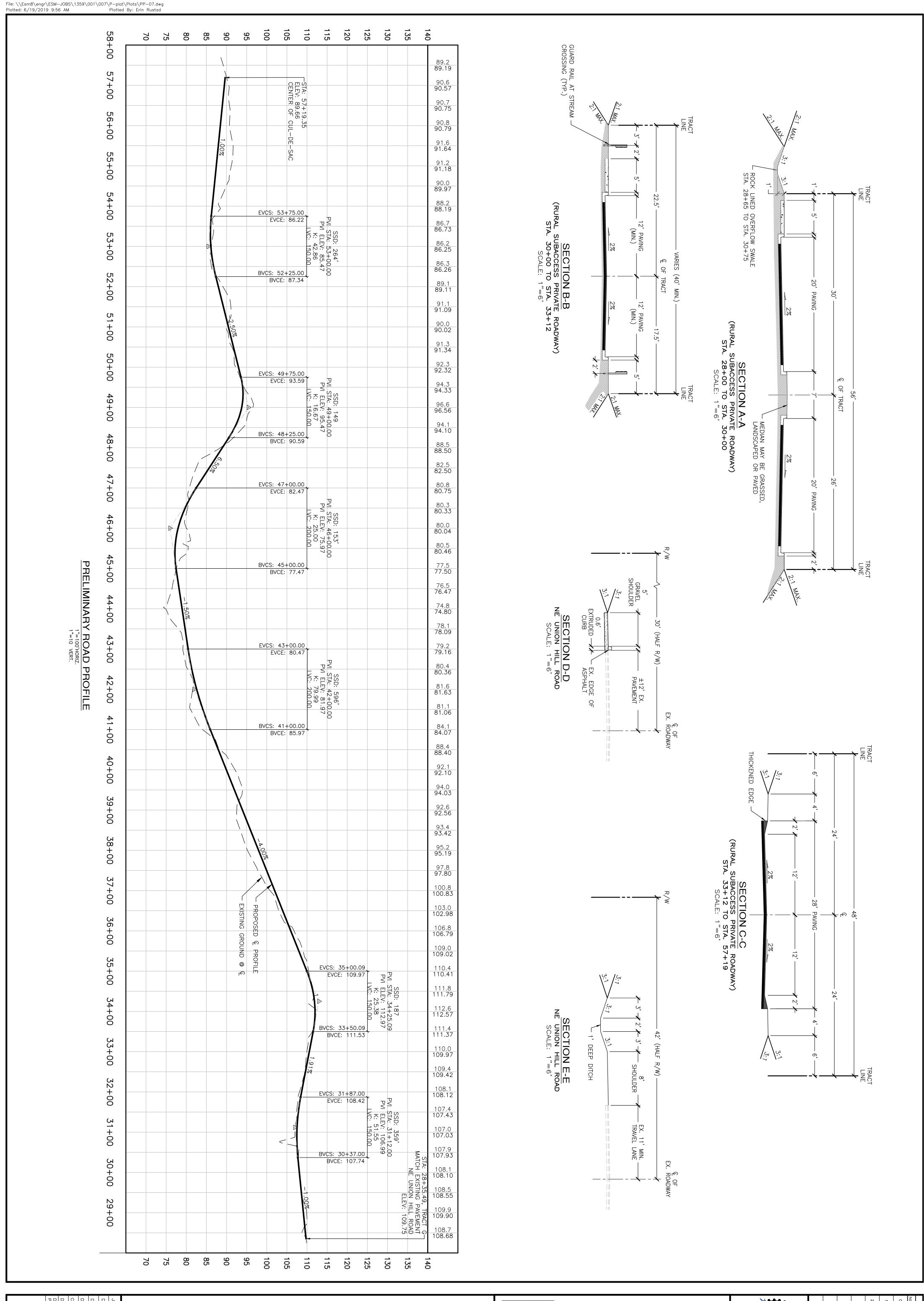
DWG. NAME: DESIGNED BY: DRAWN BY: HECKED BY: DATE OF



DESCRIPTION/DATE FIRST SUBMITTAL 05/29/2018 SECOND SUBMITTAL 11/30/2018 THIRD SUBMITTAL 06/20/2019







CONSULTING ENGINEERS
33400 8th Ave S, Suite 205
Federal Way, WA 98003 ESM THE ESTATE OF BARBARA J. NELSON, ET AL. (253) 838-6113 (425) 297-9900 FEDERAL WAY EVERETT **GUNSHY MANOR** www.esmcivil.com Civil Engineering Public Works Land Surveying Project Management Land Planning Landscape Architecture KING COUNTY PRELIMINARY ROADWAY PROFILE & CROSS SECTIONS WASHINGTON